Seismic Analysis of Multistoried Building on Sloping Ground with Ground, Middle & Top Soft Story

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Abstract: The hilly region are more prone to seismic activity. Due to scarcity of plain ground on hilly regions building are required to be constructed on sloping ground Earthquake has a high potential to cause a wide spread damage in densely populated areas which causes heavy loss of human life and high economic losses. This cause of damage is due to lack of knowledge of engineers and hence resulting in improper design of structures. Hence, it is necessary to ensure adequacy of the structures against horizontal earthquake effects. The static & dynamic analysis is carried out using the parameters like fundamental time period, storey displacement and base shear has been studied for buildings of different angles. As per codal provisions, a soft story is defined as the story in which the stiffness is less than 70% of the story above or below, in soft story building a story is kept open for the purpose vehicle parking, shops, for service etc., . This paper highlights the importance of explicitly recognizing the presence of the open storey in the analysis of the building, for that purpose modeling of G+16 story regular building is done ,software like ETABS & SAP 2000 have been used and the results are compared .

IndexTerms - EQX, EQY, RSX, RSY, GMT soft story, shear wall, sloping ground.

I. INTRODUCTION

INDIA is having the record of disastrous earthquake, which left behind loss of many lives & intense damage to assets and financial system . Analysis of structure in hill region is rather different than that of a level ground, since the column of sloped building rests @ different level . The term earthquake may be describe as any quite unstable waves which can be either natural or initiated by humans that generate unstable waves. Earthquakes are caused normally by rupture of geologic faults. Mass destruction of the low and high rise buildings in the recent earthquakes leads to the need of investigation especially in a developing country like INDIA. Structure subjected to seismic/earthquake forces are always vulnerable to damage and if it occurs on a sloped building as on hills which is at some inclination to the ground the chances of damage increases, The economic growth and rapid urbanization in hilly region has accelerated the real estate development. Due to this, population density in the hilly region has increased enormously. Therefore, there is popular and pressing demand for the construction of multi-storey buildings on hill slope in and around the cities. A soft story is characterized by vertical discontinuity in stiffiness. When an individual storey in a building is made taller and more open in construction it is called soft storey. From the past earthquake it has been observed that a building with discontinuity in the stiffness and mass subjected to concentration of forces and deformations at the point of discontinuity which may leads to the failure of members at the junction and collapse of building. Most economical way to eliminate the failure of soft storey is by adding shear walls to the tall buildings. Whenever there is requirement for a multistorey building to resist higher value of seismic forces, lateral load resisting system such as shear wall should be introduced in a building. Vertical plate like RC wall introduced in building in addition to beam, column and slab are called shear wall. Shear walls are incorporated in building to resist lateral Forces and support the gravity loads. RC shear wall has high in plane stiffness. Positioning of shear wall has influence on the overall behaviour of the building.

II. OBJECTIVES

To make out the effect of infill in frame when subjected to seismic loading. To know the behavior of the building with ground, middle and top soft-storey. To study the Seismic response of soft story structure with shear wall. To match up to results of ETABS 2016 with SAP 2000.

III. METHODOLOGY

The building has five bays in X direction with spacing of 6m and nine bays in Y direction with spacing of 4m. The plan dimension 30 m × 36 m. Typical Story height is 3.0 m for each floor including intermediate soft storey the heigt of ground and top soft storey is different. In the top storey swimming pool is modelled . . Following data is used in the analysis of the RC frame building models. Thickness of masonary wall (tw) = 0.23m, E_{masonary} = 3500000 kN/m² Density of brick masonry 20kN/m³ thickness of slab (ts) = 0.15m Density of Reinforced Concrete 25 kN/m³, Beam dimension 230 x 600 mm, Thickness of shear wall 0.20m, grade of concrete used M35 Poisson's Ratio of concrete 0.2, Floor finishes 1.0kN/m², Imposed loads 3.5KN/m², Roof live 1.5 KN/ m², water pressure = 24.525 kN/m² Zone –V, Zone factor (Table2 of IS 1893-2002) – 0.36, Importance factor, I (Table 6 of IS 1893-2002) – 1.0, Response reduction factor, R (Table 7 of IS 1893-2002) – 5.00, Soil type (figure 2 of IS 1893-2002) - Type II (Medium soil) .

Models considered for analysis are

Model 1 Bare frame model, however masses of brick masonary infill walls (230 mm thick) are included.

Model 2 Building has full brick masonary infill of 230 mm thick in all the story.

Model 3 Building has no brick masonary infill in ground, middle and top story and has full brick masonry infill of 230 mm thick

Model 4 Building model is same as model 3 further, L shaped shear wall is provided at corners.

Model 5 building model is same as model 3 further, C shaped shear wall is provided at corners.

These models are analysis for 6 degree 12 degree and 18 degree ground slope

Models in ETABS

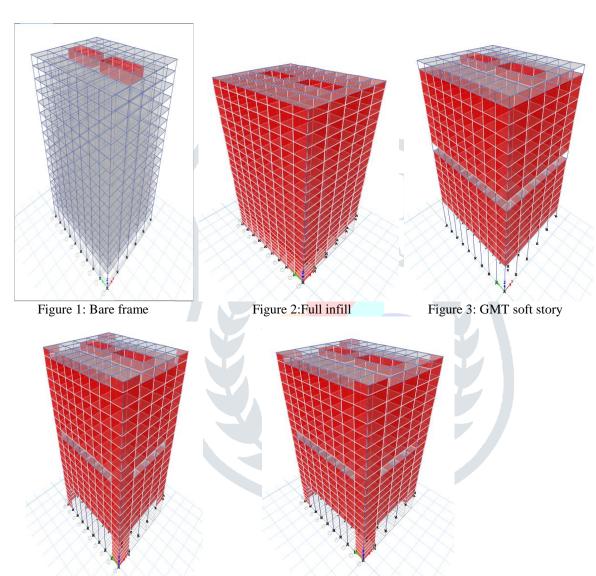


Figure 4:GMT SS & L type shear wall

Figure 5: GMT SS & C type shear wall

Figure 7:Full infill Figure 8: GMT soft story Figure 6: Bare frame

Models in SAP 2000

Figure 9:GMT SS & L type shear wall Figure 10: GMT SS & C type shear wall

IV. ANALYSIS

The analysis of building is carried out using ETABS and SAP 2000, equivalent static method and response spectrum method were used, as both methods attempt to quantitatively estimate force due to an earthquake on a structure in terms of set of static

Equivalent lateral force – seismic analysis of most of the structures are still carried out on the basis of lateral force assumed to be equivalent to the actual (dynamic) loading. This method is usually conservative for low to medium height building with a regular conformation.

Response spectrum analysis is a linear dynamic statistical analysis method which measures the contribution from each natural mode of vibration to indicate the likely maximum seismic response of an essentially elastic structure. Computer analysis can be used to determine these modes for a structure. Following are the types of combination methods in response spectrum method

- absolute peak values are added together.
- square root of the sum of the squares (SRSS) .
- complete quadratic combination (CQC) a method that is an improvement on SRSS for closely spaced modes .

V. RESULTS AND DISCUSSION

5.1 TIME PERIOD

Table shows the time period of the building. Thus it can be clearly seen that, presence of brick infill wall and shear wall considerably reduces the time period of building.

Table 5.1(a) Time Period for bare frame

Models	Time Period ETABS	Time Period SAP2000
0 degree bare frame	2.115	2.131827
6 degree bare frame	2.16	2.177733
12 degree bare frame	2.18	2.197263
18 degree bare frame	2.19	2.207461

Table 5.1(b) Time Period for full infill

Models	Time Period ETABS	Time Period SAP2000
0 degree full infill	0.57	0.594783
6 degree full infill	0.587	0.612203
12 degree full infill	0.608718	0.627968
18 degree full infill	0.631718	0.642991

Table 5.1(c) Time Period for GMT soft story

Models	Time Period ETABS	Time Period SAP2000
0 degree gmt soft story	0.614	0.643508
6 degree gmt soft story	0.655	0.685138
12 degree gmt soft storey	0.706	0.735397
18 degree gmt soft story	0.758	0.787085

Figure 11 Time Period for bare frame Vs model

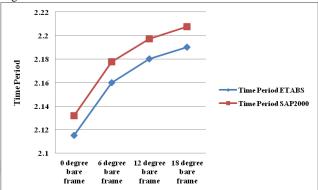


Figure 12 Time Period for full infill Vs model

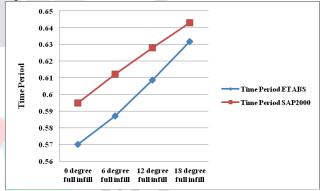


Figure 13 Period for GMT soft story Vs model

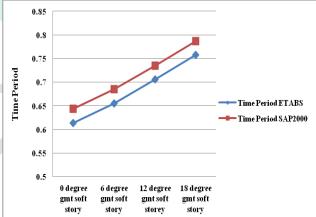


Table 5.1(d) Time Period for GMT ss & L type sw

Models	Time Period ETABS	Time Period SAP2000
0 degree gmt ss & L type sw	0.564	0.588456
6 degree gmt ss & L type sw	0.591	0.61615
12 degree gmt ss & L type sw	0.615	0.640531
18 degree gmt ss & L type sw	0.636	0.661754

Table 5.1(e) Time Period for GMT ss & C type sw

Models	Time Period ETABS	Time Period SAP2000		
0 degree gmt ss & C type sw	0.54	0.561555		
6 degree gmt ss & C type sw	0.563	0.586022		
12 degree gmt ss & C type sw	0.5845	0.608529		
18 degree gmt ss & C type sw	0.6051	0.629054		

Figure 14 Time Period for GMT ss & L type sw Vs model

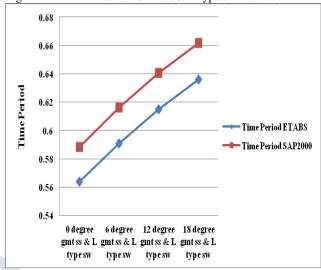


Figure 15 Time Period for GMT ss & C type sw Vs model

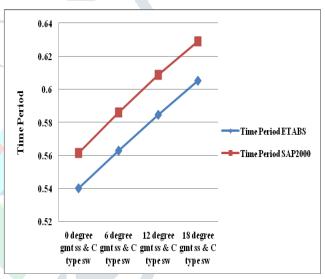


Table 5.1(f) % increase in time period of ETABS with respect to SAP 2000					
bare frame	full infill	gmt soft story	gmt ss & L type shear wall	gmt ss & C type shear wall	
0.795602837	4.347895	4.80586319	4.33617	3.991666667	
0.820972222	4.293526	4.60122137	4.255499	4.089165187	
0.791880734	3.162384	4.16388102	4.151382	4.111035073	
0.797305936	1.784499	3.83707124	4.049371	3.958684515	

5.2 Story Displacement

The maximum displacements at roof level with respect to ground are presented in Tables for Equivalent static and Response spectrum method. For better comparability the displacement for each model along the two directions of ground motion are plotted in graphs as shown in Figures . The floor displacement will be maximum at the top floor, gradually reducing down the height of the building to an almost negligible displacement at the lowest basement floor. Buildings resting on sloping ground has more displacement compared to buildings on Plain ground and the presence of Shear wall reduces the lateral displacement considerable by both Equivalent static and Response spectrum analysis.

Table 5.2(a) max story displacement bare frame

Model	EQX ETABS	EQX SAP2000	EQY ETABS	EQY SAP2000
0 degree bare frame	53.717	54.23167	46.472	46.9182
6 degree bare frame	54.098	54.39862	47.306	47.56251
12 degree bare frame	53.563	53.7643	47.367	47.53824
18 degree bare frame	52.974	53.08452	47.306	47.39776

Table 5.2(b) max story displacement bare frame

Model	RSX ETABS	RSX SAP2000	RSY ETABS	RSY SAP2000
0 degree bare frame	41.787	41.90662	36.849	37.00106
6 degree bare frame	42.422	42.27246	41.066	38.67805
12 degree bare frame	42.827	43.09476	41.255	41.64087
18 degree bare frame	43.197	43.43618	40.589	40.93949

Table 5.2(c) max story displacement full infill

Model	EQX ETABS	EQX SAP2000	EQY ETABS	EQY SAP2000
0 degree full infill	12.977	13.55078	14.463	14.90 <mark>256</mark>
6 degree full infill	13.857	14.17908	15.157	15.49085
12 degree full infill	14.717	14.28575	16.72	15.73753
18 degree full infill	14.414	14.39103	15.763	15.97448

Table 5.2(d) max story displacement full infill

Model	RSX ETABS	RSX SAP2000	RSY ETABS	RSY SAP2000
0 degree full infill	11.215	11.83549	12.496	12.92937
6 degree full infill	12.157	12.57437	13.681	14.02749
12 degree full infill	13.281	12.87641	16.117	14.81461
18 degree full infill	13.078	13.19261	15.346	15.5608

Figure 16 max story displacement bare frame Vs model

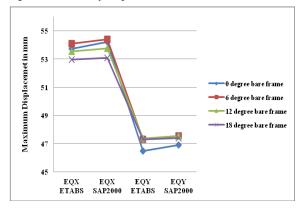


Figure 17 max story displacement bare frame Vs model

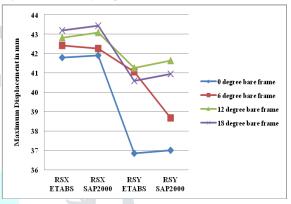


Figure 18 max story displacement full infill Vs model

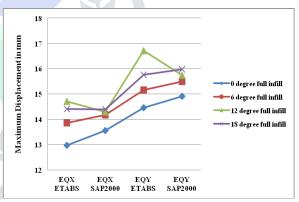


Figure 19 max story displacement full infill Vs model

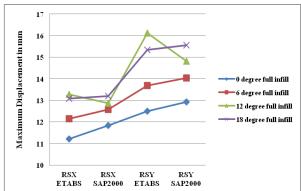


Table 5.2(e) max story displacement GMT soft storey

Model	EQX ETABS	EQX SAP2000	EQY ETABS	EQY SAP2000
0 degree gmt soft story	14.293	14.61285	15.274	15.87088
6 degree gmt soft story	14.617	14.88671	16.114	16.67098
12 degree gmt soft story	15.766	14.96081	17.121	17.26336
18 degree gmt soft story	14.794	15.00983	17.248	17.76839

Table 5.2(f) max story displacement GMT soft storey

Model	RSX ETABS	RSX SAP2000	RSY ETABS	RSY SAP2000
0 degree gmt soft story	12.272	12.63385	13.131	13.68306
6 degree gmt soft story	12.63	12.94307	15.556	16.01755
12 degree gmt soft story	14.046	13.17141	17.75	18.17675
18 degree gmt soft story	13.139	13.41252	19.277	19.70681

Table 5.2(g) max story displacement GMT & L type sw

Model	EQX ETABS	EQX SAP2000	EQY ETABS	EQY SAP2000
0 degree gmt ss & L type sw	12.214	12.78773	14.204	14.70028
6 degree gmt ss & L type sw	13.023	13.56926	14.774	15.22175
12 degree gmt ss & L type sw	14.665	13.65053	16.376	15.53039
18 degree gmt ss & L type sw	13.52	13.66898	15.373	15.7767

Table 5.2(h) max story displacement GMT & L type sw

Model	RSX ETABS	RSX SAP2000	RSY ETABS	RSY SAP2000
0 degree gmt ss & L type sw	10.542	11.13458	12.283	12.74587
6 degree gmt ss & L type sw	11.365	11.93979	13.318	13.73476
12 degree gmt ss & L type sw	12.878	12.18967	15.6	14.59886
18 degree gmt ss & L type sw	12.18	12.40661	15.018	15.38957

Figure 20 max story displacement GMT soft storey Vs model

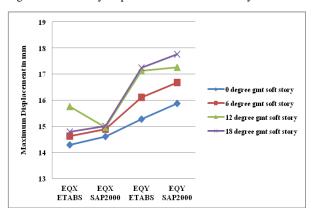


Figure 21 max story displacement GMT soft storey Vs model

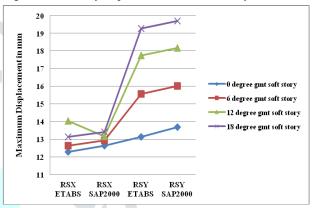


Figure 22 max story displacement GMT & L type sw Vs model

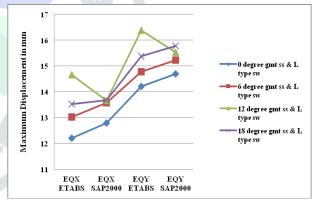


Figure 23 max story displacement GMT & L type sw Vs model

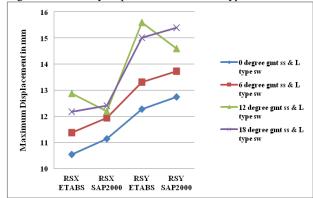


Table 5.2(i) max story displacement GMT & C type sw

Table 3.2(1) max story displacement GWT & C type sw				
Model	EQX ETABS	EQX SAP2000	EQY ETABS	EQY SAP2000
0 degree gmt ss & C type sw	12.036	12.59625	13.679	14.25062
6 degree gmt ss & C type sw	12.842	13.37404	14.318	14.7327
12 degree gmt ss & C type sw	13.498	13.56863	15.105	15.00856
18 degree gmt ss & C type sw	13.446	13.58939	14.86	15.23475

Table 5.2(j) max story displacement GMT & C type sw

Model	RSX ETABS	RSX SAP2000	RSY ETABS	RSY SAP2000
0 degree gmt ss & C type sw	10.385	10.96045	11.882	12.42284
6 degree gmt ss & C type sw	11.203	11.76175	13.029	13.42581
12 degree gmt ss & C type sw	11.961	12.11372	14.207	14.28512
18 degree gmt ss & C type sw	12.113	12.33219	14.707	15.06296

Figure 24 max story displacement GMT & C type sw Vs model

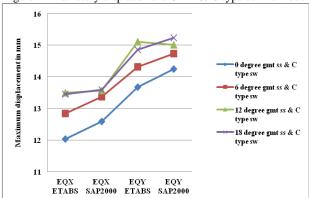


Figure 25 max story displacement GMT & C type sw Vs model

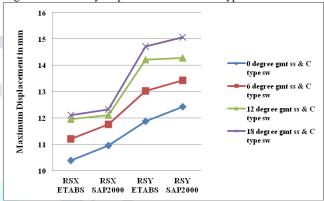


Table 5.2(k) % ↑ or ↓ values for ETABS EQX wrt SAP2000					
Model	0 degree	6 degree	12 degree	18 degree	
Bare Frame	0.958114	0.555701	0.375819	0.208631	
Full infill	4.421515	2.324334	-2.93028	-0.15936	
GMT soft story	2.237809	1.845146	-5.10713	1.458902	
GMT soft story & L type sw	4.697315	4.194594	-6.91763	1.101923	
GMT soft story & C type sw	4.654786	4.142992	0.523263	1.066414	

Table 5.2(1)% ↑ or ↓ values for ETABS EQY wrt SAP2000					
Model	0 degree	6 degree	12 degree	18 degree	
Bare Frame	0.960148	0.542227	0.361518	0.193971	
Full infill	3.039203	2.202593	-5.87602	1.341623	
GMT soft story	3.907817	3.456485	0.831493	3.017103	
GMT soft story & L type sw	3.493945	3.030648	-5.16372	2.626033	
GMT soft story & C type sw	4.178814	2.896375	-0.63846	2.521871	

Table 5.2(m)% ↑ or ↓ values for ETABS RSX wrt SAP2000					
Model	0 degree	6 degree	12 degree	18 degree	
Bare Frame	0.286261	-0.35251	0.625213	0.553696	
Full infill	5.532679	3.433166	-3.04638	0.876357	
GMT soft story	2.948582	2.478781	-6.22661	2.081741	
GMT soft story & L type sw	5.621135	5.057545	-5.34501	1.860509	
GMT soft story & C type sw	5.541165	4.987503	1.276816	1.809543	

Table 5.2(n)% ↑ or ↓ values for ETABS RSY wrt SAP2000						
Model	0 degree 6 degree 12 degree 18 degree					
Bare Frame	0.412657	-5.81491	0.935329	0.86351		
Full infill	3.46807	2.532637	-8.08085	1.399713		
GMT soft story	4.204249	2.967022	2.404225	2.229652		
GMT soft story & L type sw	3.768379	3.129299	-6.41756	2.474164		
GMT soft story & C type sw	4.551759	3.045591	0.54987	2.420344		

5.3 Base Shear

The base shear for Equivalent static method (Vb) and the Response spectrum method (VB) as per IS 1893: 2002(Part I) for various building models are listed in the tables below. Results are compared between ETABS and SAP 2000.

Table 5.3(a) Base Shear bare frame

Model	Base Shear EQX ETABS	Base Shear EQX SAP2000	Base Shear EQY ETABS	Base Shear EQY SAP2000
0 degree bare frame	6643.912	6667.044	7700.076	7772 <mark>.794</mark>
6 degree bare frame	6663.194	6699.984	7607.591	7648.283
12 degree bare frame	6680.663	6705.207	7500.284	7526.507
18 degree bare frame	6728.484	6741.501	7457.94	7471.013

Figure 26 Base Shear bare frame Vs model

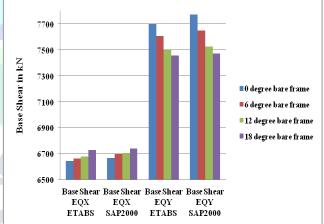


Table 5.3(b) Base Shear bare frame

Model	Base Shear RSX ETABS	Base Shear RSX SAP2000	Base Shear RSY ETABS	Base Shear RSY SAP2000
0 degree bare frame	6744.346	6786.704	7899.696	7923.337
6 degree bare frame	6791.985	6829.749	7751.604	7796.415
12 degree bare frame	6809.557	6835.074	7645.271	7672.281
18 degree bare frame	6858.755	6872.071	7602.092	7615.712

Figure 27 Base Shear bare frame Vs model

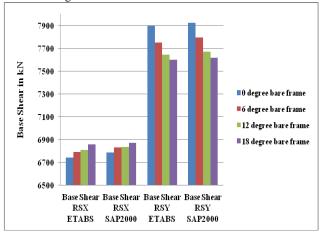


Table 5.3(c) Base Shear full infill

Model	Base Shear EQX ETABS	Base Shear EQX SAP2000	Base Shear EQY ETABS	Base Shear EQY SAP2000
0 degree full infill	28232.57	28683.42	26945.45	26234.4
6 degree full infill	29605.65	29482.76	27456.43	26503.1
12 degree full infill	33701.49	33350.24	30649.24	29311.1
18 degree full infill	30162.34	29312.17	27344.9	26178.76

Table 5.3(d) Base Shear full infill

Model	Base Shear RSX ETABS	Base Shear RSX SAP2000	Base Shear RSY ETABS	Base Shear RSY SAP2000
0 degree full infill	28781.01	29238.96	27470.91	26742.5
6 degree full infill	30177.96	30053.79	27987.08	27016.41
12 degree full infill	34353.27	34018.7	31241.09	30520.7
18 degree full infill	30744.88	29879.89	27872.69	26685.78

Table 5.3(e) Base Shear GMT soft story

Model	Base Shear EQX ETABS	Base Shear EQX SAP2000	Base Shear EQY ETABS	Base Shear EQY SAP2000
0 degree gmt soft story	25816.23	25484.73	23724.04	23022.7
6 degree gmt soft story	24985.19	24596.95	22763.84	22051.51
12 degree gmt soft story	24266.19	23874.58	21363.78	20737.98
18 degree gmt soft story	23830.77	23425.83	20111.83	19564.78

Figure 28 Base Shear full infill Vs model

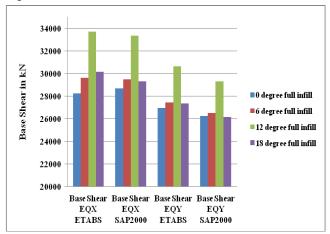


Figure 29 Base Shear full infill Vs model

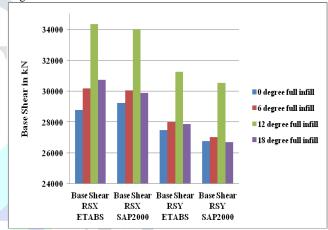


Figure 30 Base Shear GMT soft story Vs model

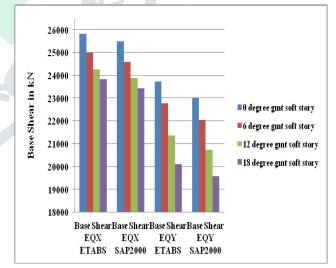


Table 5.3(f) Base Shear GMT soft story

Model	Base Shear RSX ETABS	Base Shear RSX SAP2000	Base Shear RSY ETABS	Base Shear RSY SAP2000
0 degree gmt soft story	26315.23	25978.31	24182.91	23468.61
6 degree gmt soft story	25469.04	25073.35	23204.14	22478.61
12 degree gmt soft story	24735.61	24336.98	21776.73	21139.63
18 degree gmt soft story	24292.12	23879.54	20499.82	19943.71

Table 5.3(g) Base Shear GMT soft story & L type sw

Model	Base Shear EQX ETABS	Base Shear EQX SAP2000	Base Shear EQY ETABS	Base Shear EQY SAP2000
0 degree gmt ss & L type sw	26974.14	27424.98	25996.1	25353.11
6 degree gmt ss & L type sw	27670.73	28008.77	25454.91	24729
12 degree gmt ss & L type sw	28003.87	27519.53	24753.76	24031.41
18 degree gmt ss & L type sw	27638.84	27157.36	24233.82	23506.79

Table 5.3(h) Base Shear GMT soft story & L type sw

Model	Base Shear RSX ETABS	Base Shear RSX SAP2000	Base Shear RSY ETABS	Base Shear RSY SAP2000
0 degree gmt ss & L type sw	27496.22	27956.15	26499.01	25844.15
6 degree gmt ss & L type sw	28205.18	28551.25	25946.46	25207.95
12 degree gmt ss & L type sw	28544.31	28764.41	25232.78	24496.85
18 degree gmt ss & L type sw	28172.41	27683.35	24702.91	23962.07

Figure 31 Base Shear GMT soft story Vs model

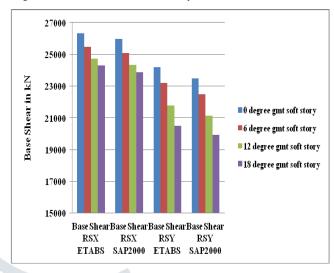


Figure 32 Base Shear GMT soft story & L type sw Vs model

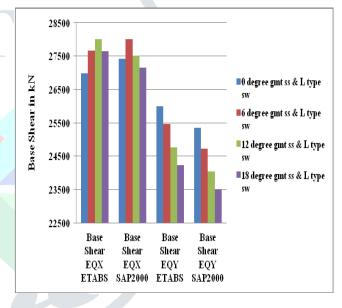


Figure 33 Base Shear GMT soft story & L type sw Vs model

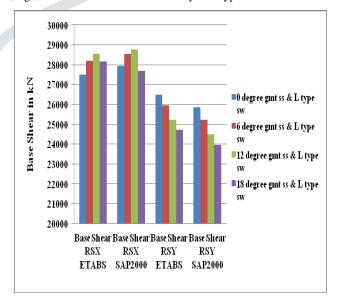


Table 5.3(i) Base Shear GMT soft story & C type sw

Table 5.5(1) Base Blicar GWT soft story & C type sw					
Model	Base Shear EQX ETABS	Base Shear EQX SAP2000	Base Shear EQY ETABS	Base Shear EQY SAP2000	
0 degree gmt ss & C type sw	27050.53	27501.37	27050.53	26641.62	
6 degree gmt ss & C type sw	27771.06	28106.84	26819.51	26091.38	
12 degree gmt ss & C type sw	32054.64	32858.32	25577.12	25391.19	
18 degree gmt ss & C type sw	27996.26	27506.29	24988.19	24829.99	

Figure 34 Base Shear GMT soft story & C type sw Vs model

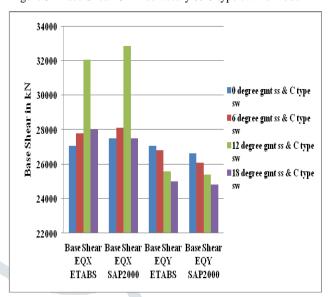


Table 5.3(j) Base Shear GMT soft story & C type sw

	Base	Base	Base	Base
	Shear	Shear	Shear	Shear
	RSX	RSX	RSY	RSY
Model	ETABS	SAP2000	ETABS	SAP2000
0 degree gmt				
ss & C type				
sw	27575.01	28034.02	27574.27	27157.62
6 degree gmt				
ss & C type				
sw	28308.41	28651.21	27337.53	26596.71
12 degree				
gmt ss & C				
type sw	32674.69	33397.88	26336.86	25882.97
18 degree				
gmt ss & C				
type sw	28537.52	28039.04	26083.16	25310.9

Figure 35 Base Shear GMT soft story & C type sw Vs model

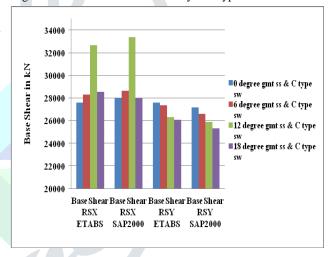


Table 5.3(k) % ↑ or ↓ values for ETABS EQX wrt SAP2000						
Model	0 degree	6 degree	12 degree	18 degree		
Bare Frame	0.348168	0.552138	0.367389	0.193461		
Full infill	1.596914	-0.41509	-1.04224	-2.81865		
GMT soft story	-1.28408	-1.55388	-1.61381	-1.69923		
GMT soft story & L type sw	1.671379	1.221652	-1.72955	-1.74204		
GMT soft story & C type sw	1.666659	1.2091	2.507219	-1.75013		

Table 4.3(1) $\% \uparrow$ or \downarrow values for ETABS EQY wrt SAP2000					
Model	0 degree	6 degree	12 degree	18 degree	
Bare Frame	0.94438	0.534887	0.349627	0.17529	
Full infill	-2.63885	-3.47216	-4.36598	-4.26456	
GMT soft story	-2.95624	-3.12922	-2.92926	-2.72004	
GMT soft story & L type sw	-2.47341	-2.85175	-2.91814	-3.00006	
GMT soft story & C type sw	-1.51165	-2.71493	-0.72694	-0.6331	

Table 5.3(m) % ↑ or ↓ values for ETABS RSX wrt SAP2000						
Model	0 degree	6 degree	12 degree	18 degree		
Bare Frame	0.628052	0.556008	0.374723	0.194146		
Full infill	1.591153	-0.41146	-0.97391	-2.81344		
GMT soft story	-1.28032	-1.55361	-1.61156	-1.69841		
GMT soft story & L type sw	1.672703	1.226973	0.771082	-1.73595		
GMT soft story & C type sw	1.6645869	1.2109476	2.2133033	-1.746753		

Table 5.3(n) % ↑ or ↓ vaues for ETABS RSY wrt SAP2000						
Model	0 degree	6 degree	12 degree	18 degree		
Bare Frame	0.299265	0.578087	0.35329	0.179161		
Full infill	-2.65157	-3.46828	-2.30591	-4.25833		
GMT soft story	-2.95374	-3.12673	-2.9256	-2.71276		
GMT soft story & L type sw	-2.47126	-2.84628	-2.91656	-2.999		
GMT soft story & C type sw	-1.51101	-2.709901	-1.723402	-2.960761		

VI. Conclusions

Fundamental time period decreases when effect of masonry infill and shear wall is considered .

Bare frame model has got highest time period compared to other models & C type shear wall building with soft story has got least value of time period.

Buildings resting on sloping ground have more lateral displacement compared to buildings on Plain ground, as the angle of slope was increased displacement were also reduced, and the presence of Shear wall & infill reduces the lateral displacement of the soft story building.

Bare frame model has got the least base shear compared to other models. Base shear was increased in X direction and decreased in Y direction.

A attempt to compare the results of ETABS with SAP2000 is made, the result obtained for time period max story displacement and maximum storey drift & base shear are nearly same hence modeling & analyzing in either of the structural software can be done.

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REFERENCES

- [1] Rayyan-Ul-Hasan Siddiqui 1, H. S. Vidyadhara1Post Graduate Student, Department of Civil Engineering, Poojya Doddappa AppaCollege of Engineering, Gulbarga 585-104, INDIA2Associate Professor, Department of Civil Engineering, Poojya Doddappa Appa College of Engineering, Gulbarga 585-104, INDIA: Seismic Analysis of Earthquake Resistant Multi Bay Multi Storeyed 3D - RC Frameinterrnational Journal of Engineering Research & Technology (IJERT)Vol. 2 Issue 10, October – 2013
- [2] Prasad Ramesh Vaidya International Journal of Civil and Structural Engineering Research ISSN 2348-7607 (Online) Vol. 2, Issue 2, pp: (53-60), Month: October 2014 - March 2015, Available at: www.researchpublish.com Page | 53 Research Publish Journals Seismic Analysis of Building with Shear Wall on Sloping Ground .
- [3] ETABS "Extended 3-D analysis of the building systems", California Computers and structures Inc. Berkeley
- [4] From Start To Finish: Model, Analysis, Design Of Multi-story Concrete structures using ETABS. CSI Educational Services Computers and Structures ,Inc1995 University Avenue Berkeley , California , USA .
- [5] SAP2000 "Structural Analysis Programme", Computers and Structures inc, Berkeley.
- [6] IS456:2000. "Indian Standard Code of Practice for Plain and reinforced concrete", Bureau of Indian Standards, New Delhi.
- [7] IS 875:2000. "Code of practice for design loads part2 :Imposed Loads", Bureau of Indian Standards, New Delhi .
- [8] IS 1893-2000. Criteria for Earthquake Resistant Design Of Structures, Part-1 General Provision and Building. Bureau of Indian Standards, New Delhi.
- [9] IS 13920:1993."Indian Standard Code of the practice for the detailing of Reinforced Concrete Structures subjected to seismic Forces" Bureauo of Indian Standards.

