

PARAMETRIC STUDY OF DIAGRID STRUCTURES SUBJECTED TO SEISMIC FORCES

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Abstract: Parametric study of diagrid structure has been carried out by analyzing and designing 40 storey diagrid structure having variation in parameters like angle of diagrid, cross sectional shape of diagrid and column placed at different position. The plan area considered was 24m * 24m (aspect ratio equals to 5). The parameters nominated for evaluation are 3 different angle i.e. 45 degree, 63.43 degree and 78.69 degree. The second parameter considered was 4 different cross sectional shape i.e. I section, box section, tube section and composite section made up of tube section having concrete as an infill material. And the last parameter considered was the 3 different position of column i.e. no column placed on the periphery, column placed at the corner of the plan and column placed at the center of the external edge. Finally the results generated due to the variation of the above parameters were compared in terms of maximum top storey displacement, storey drift and base shear.

Index Terms – Parametric Study, I section, Box section, Tube section, Composite section, 45 degree, 63.43 degree, 78.69 degree, column position 1 N.A., column position 2 corner, column position 3 center.

I. INTRODUCTION

Scarcity of land restricted the horizontal development and resulted in evolution of vertical growth of the town which leads to the development of tall buildings. The concept of tall structures was introduced by Fazlur Khan in early 60's. The different structural systems provided start-up to the tall structures while advances in material and construction technology accelerated the development of tall structures. The Lateral load due to wind and earthquake is the major factor that governs the design of tall structures which are mainly resisted either by exterior or interior structural system.

The different type of lateral load resisting systems that are widely used are mainly

- Rigid frame
- Shear wall
- Braced tube system
- Exoskeleton
- Diagrid system
- Hexa-grid system
- Tube in tube system and
- Bundled tube system
- Core-and- outrigger
- Belt Truss
- Staggered Truss

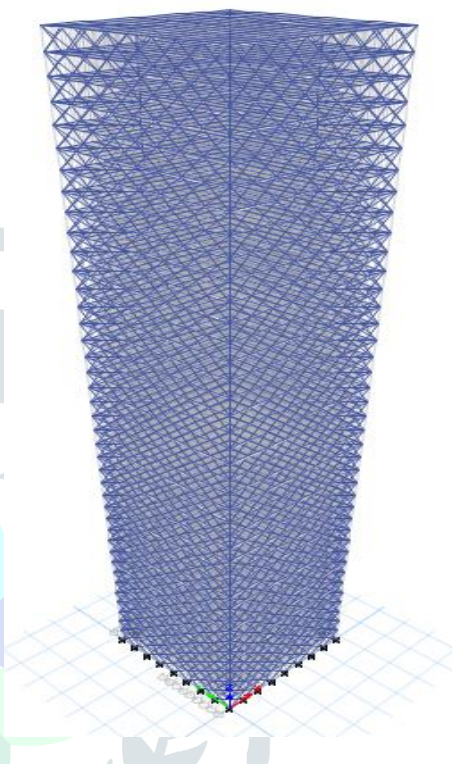
Diagrid is an exterior structural system in which almost all vertical columns can be eliminated and consists of only inclined columns on the facade of the building having triangular configuration. The major difference between a braced tube building and a diagrid building is that, there are no vertical columns present in the perimeter of a diagrid building. Diagrid structures do not need high shear rigidity cores because shear can be carried by the diagrid located on the perimeter. The diagonal members in diagrid structural system can carry both gravity as well as lateral load. The structural efficiency of diagrid system helps in avoiding interior and corner columns which allows architectural flexibility with the floor plan. The openings between the diagonal members can be filled either by glass or any aesthetically sound material. Now a days the concept of curtain wall is adopted to fill this large openings between diagonal members. Diagrid structural system saves approximately 20% structural steel weight when compared to a conventional moment frame structure. An early example of diagrid structure is the IBM building in Pittsburgh built in early 1960s, with its 13 storey building height. Some other well-known examples of diagrid structures are Hearst tower (New York), Capital Gate (Abu Dhabi), CCTV headquarters building (Beijing), West tower (Guangzhou), Mode Gakuen Spiral Tower (Aichi), The Swiss Re tower (London), Lotte super tower (Seoul), etc.

II. PROBLEM FORMULATION

For parametric study 40 storey building having base dimension 24m*24m was considered. Modelling Analysis and Design was carried out using Etabs. Software. IS 800 was consider for designing steel members. Building was analyzed and designed for dead load, live load, floor finish, wall load and earth quake load in both the direction. Zone 5 was considered for extreme conditions for both static as well as response spectrum analysis of earth quake. The parameters selected for comparison are base shear, storey drift and displacement.

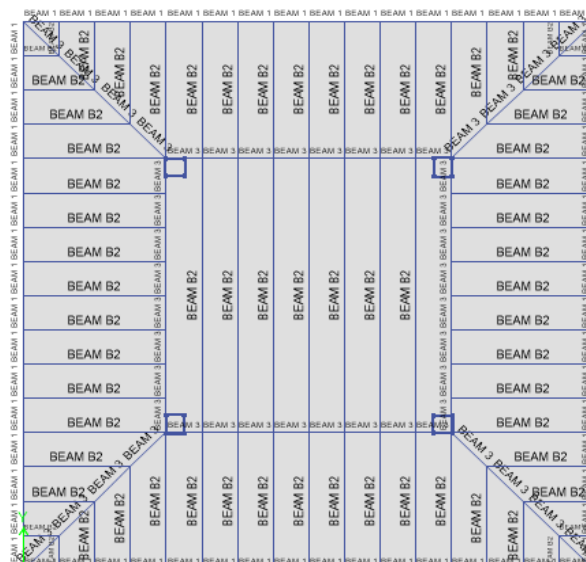
A. Building configuration

| | |
|---------------------------|------------------------------|
| Plan Area | 18 m × 18 m |
| Number of storey | 30 |
| Typical Storey Height | 3 m |
| Steel Sections | Fe 250 |
| Concrete (Slabs) | M30 |
| Live Load | 3 kN/m ² |
| Floor Finish | 1 kN/m ² |
| Wall Load | 12.42 kN/m |
| Slab Thickness | 150 mm |
| Zone Factor | zone 5 |
| Importance Factor | 1.2 |
| Response Reduction Factor | 5 |
| Analysis Method | Static and Response Spectrum |



B. Beam

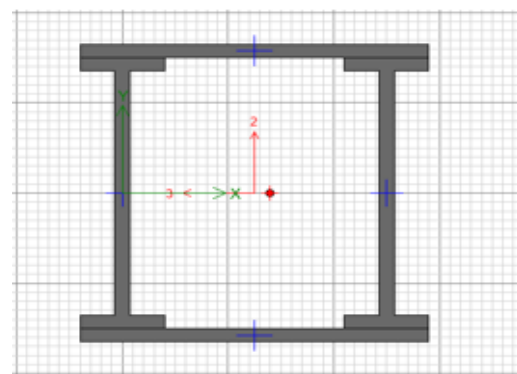
| Details/Storey | Beam 1 | Beam 2 | Beam 3 |
|---------------------------------------|---------|--------|--------|
| Total depth (H) | ISMB500 | 550 | 600 |
| Width of flange (B _f) | | 250 | 250 |
| Thickness of flange (t _f) | | 25 | 25 |
| Thickness of web (t _w) | | 15 | 15 |
| Plate dimension | | 260*25 | 260*25 |



C. Column

Column is made up of 2 I section having cover plate at top and bottom as shown in figure below.

| | | | | |
|---|----------|----------|----------|----------|
| Details/Storey | 0 to 10 | 11 to 20 | 21 to 30 | 31 to 40 |
| Total depth (H) | 1600 | 1250 | 1050 | 820 |
| Width of flange (B_f) | 415 | 315 | 275 | 210 |
| Thickness of flange (t_f) | 100 | 75 | 75 | 65 |
| Thickness of web (t_w) | 100 | 75 | 75 | 65 |
| Plate dimension | 1800*100 | 1400*75 | 1200*75 | 950*65 |
| Center to center distance b/w I section | 1385 | 1085 | 925 | 740 |



D. I section Diagrid

| | | | | |
|-------------------------------|---------|----------|----------|----------|
| Details/Storey | 0 to 10 | 11 to 20 | 21 to 30 | 31 to 40 |
| Total depth (H) | 810 | 700 | 660 | 550 |
| Width of flange (B_f) | 420 | 360 | 340 | 275 |
| Thickness of flange (t_f) | 50 | 60 | 50 | 50 |
| Thickness of web (t_w) | 30 | 30 | 30 | 30 |
| Plate dimension | 440*70 | 390*80 | 360*65 | 295*65 |

| | | | | |
|----------------|---------|----------|----------|----------|
| Details/Storey | 0 to 10 | 11 to 20 | 21 to 30 | 31 to 40 |
| Width of box | 420 | 375 | 340 | 275 |
| Depth of box | 420 | 375 | 340 | 275 |
| Thickness | 45 | 45 | 40 | 40 |

E. Box section Diagrid

F. Tube section Diagrid

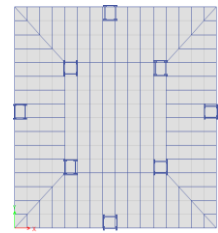
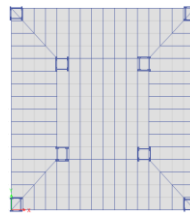
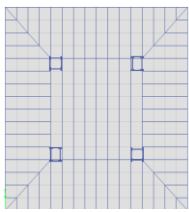
| | | | | |
|----------------|---------|----------|----------|----------|
| Details/Storey | 0 to 10 | 11 to 20 | 21 to 30 | 31 to 40 |
| Diameter | 475 | 425 | 375 | 325 |
| Thickness | 45 | 45 | 40 | 30 |

G. Composite section Diagrid

| | | | | |
|----------------|---------|----------|----------|----------|
| Details/Storey | 0 to 10 | 11 to 20 | 21 to 30 | 31 to 40 |
| Diameter | 400 | 370 | 350 | 325 |
| Thickness | 35 | 30 | 30 | 30 |

Infill material: - M20 Grade Concrete

III. Types of Model



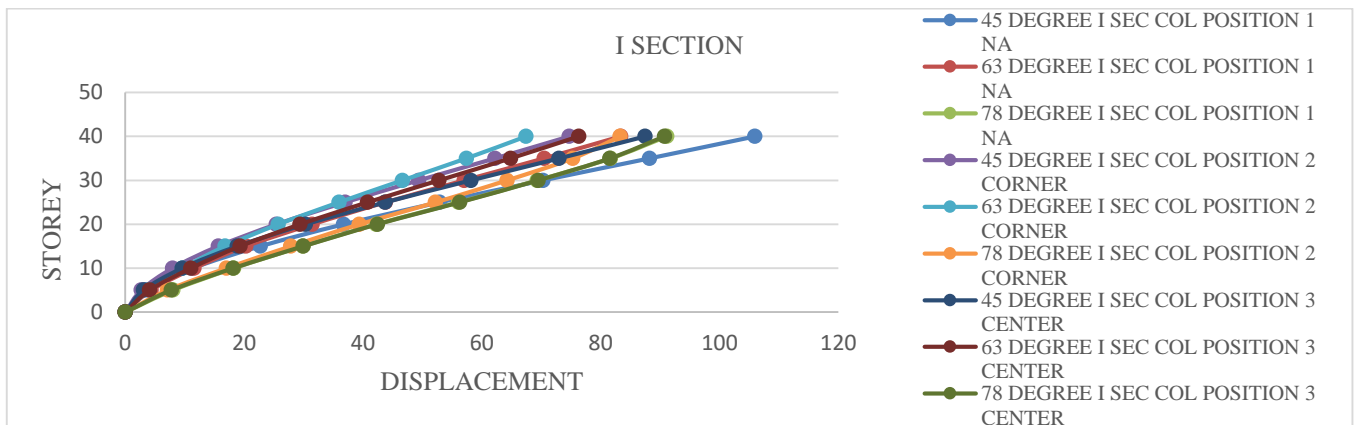
COLUMN POSITION 1 N.A.

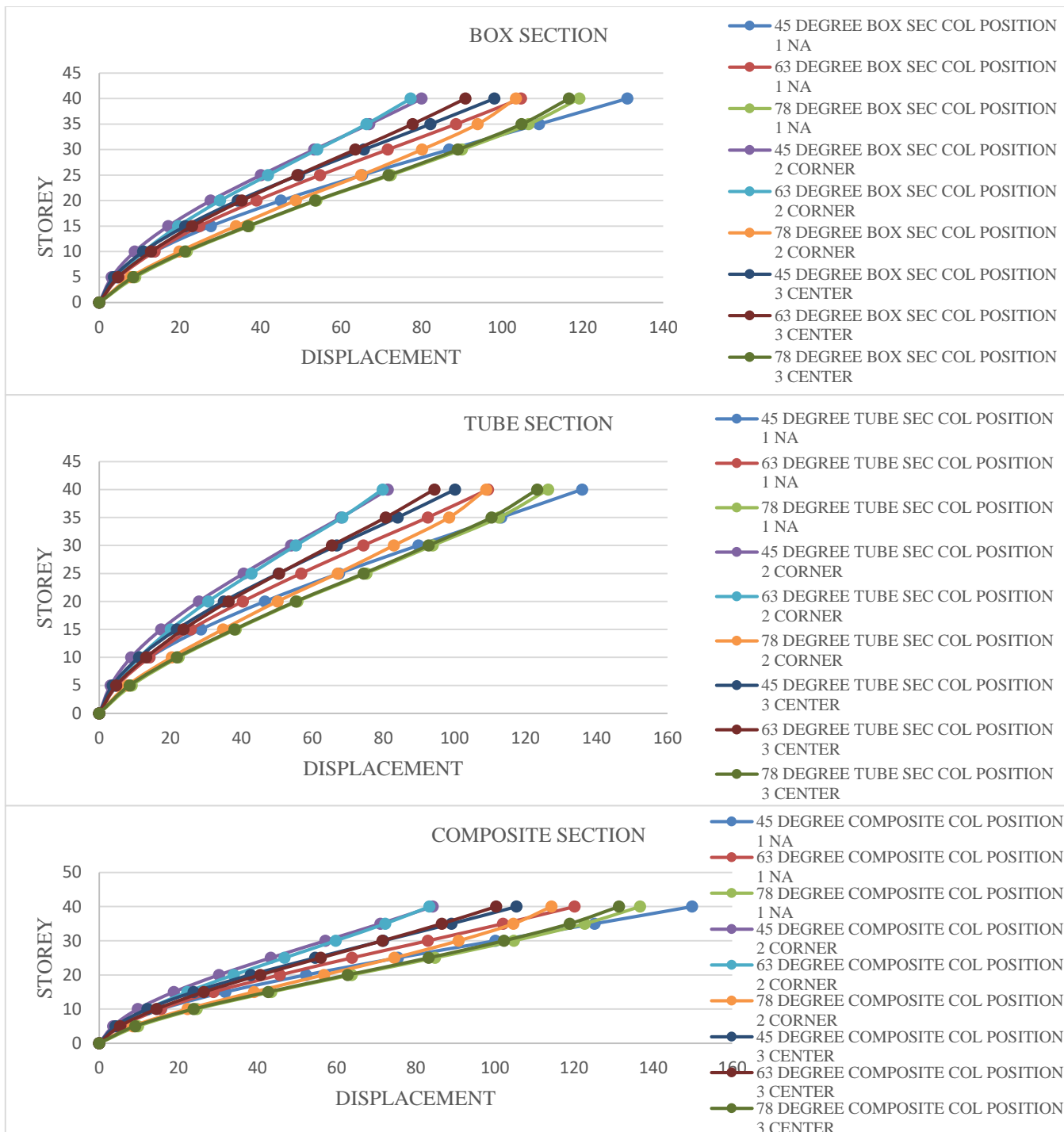
COLUMN POSITION 2 CORNER

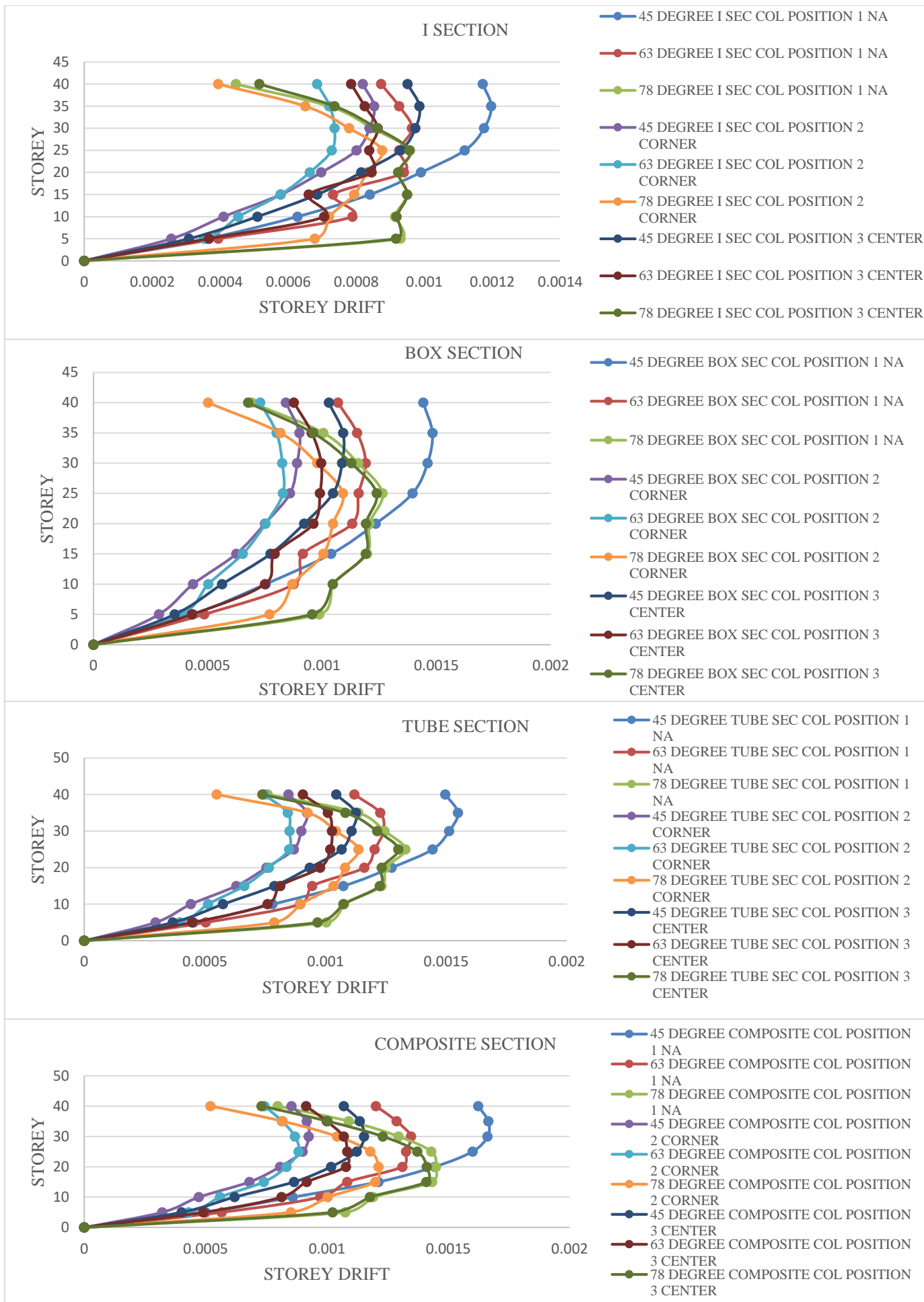
COLUMN POSITION 3 CENTER

IV. Result and

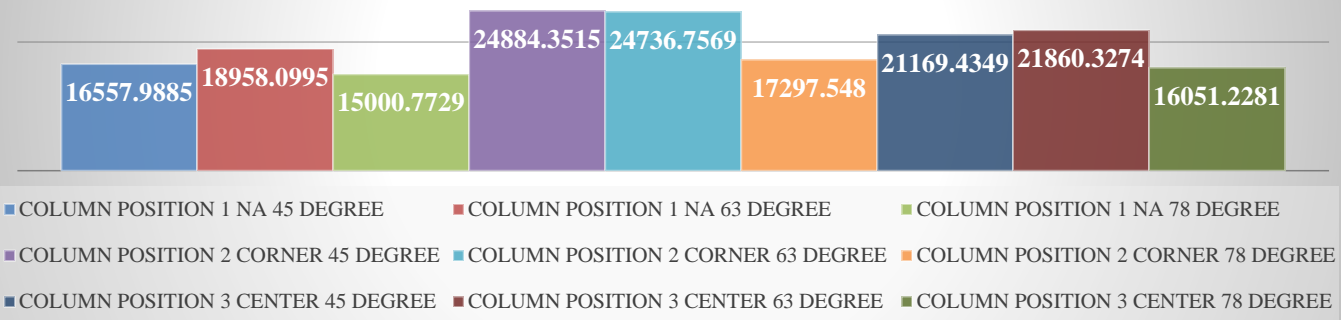
Comparison



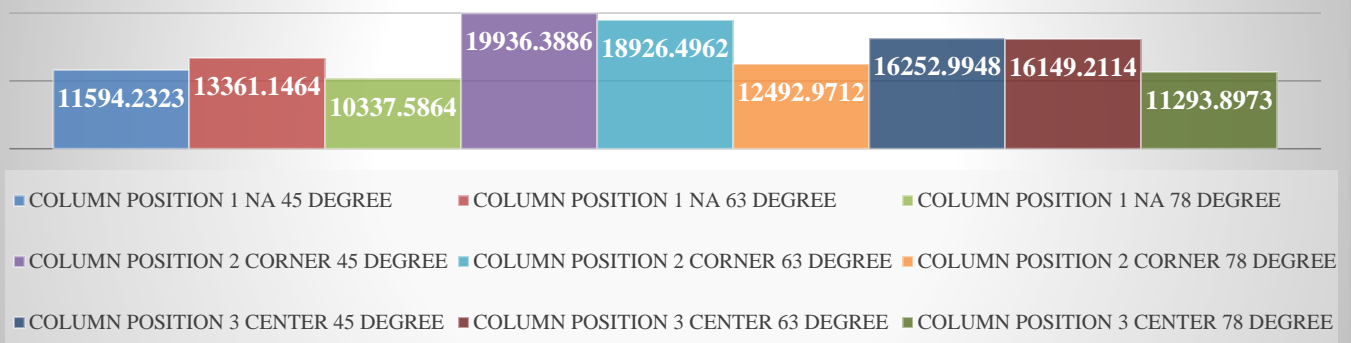




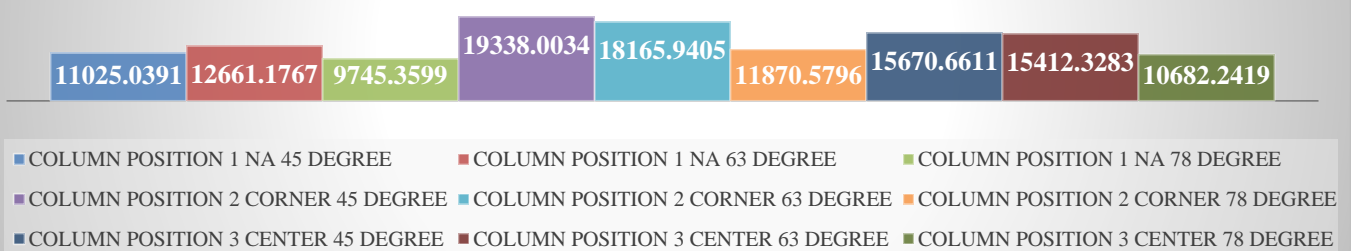
I SECTION



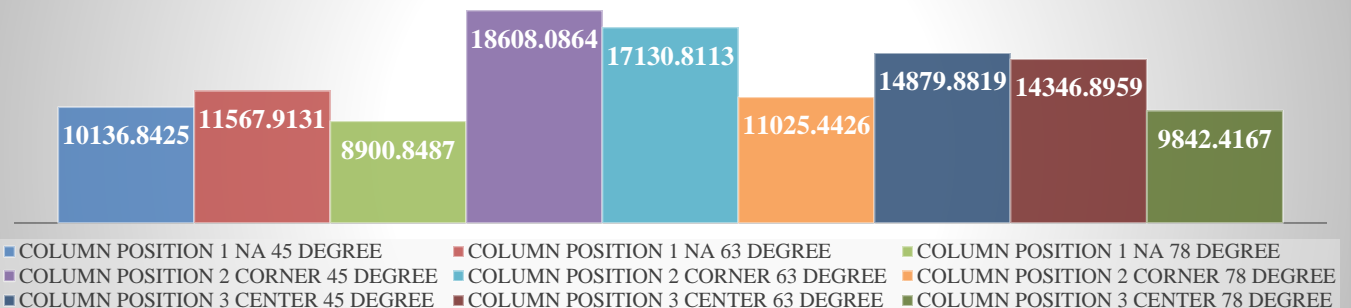
BOX SECTION



TUBE SEC



COMPOSITE SECTION



V. Conclusion

- Most efficient section: - I section
- Optimum angle: - 63 degree
- Most efficient column position: - Corner
- Column placed at corner: - not much difference is observed in base shear due to variation of angle

VI. References

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