

# EXPERIMENTAL INVESTIGATION OF EFFECT OF SAND FINES ON PROPERTIES OF SELF COMPACTING CONCRETE

<sup>1</sup>P.N.Nimodiya, <sup>2</sup>H.S.Patel

<sup>1</sup>Ph.D. Scholar, <sup>2</sup>Professor

<sup>1</sup>Civil Engineering Department, <sup>1</sup>Gujarat Technological University, Ahmedabad, Gujarat, India

<sup>2</sup>Applied Mechanics Department, <sup>1</sup>L.D.College of Engineering, Ahmedabad, Gujarat, India

**Abstract:** It is proven that fines are important constituent to produce self compacting concrete (SCC). Comparing to conventional concrete more fines are added in SCC. Cement, fly ash and any filler material finer than 125 micron are considered as powder or fines. Amount of fines affects the fresh and hardened properties of the self compacting concrete. Fine aggregate used in the production of concrete also has some fines in it. In this paper effect of sand fines on fresh and hardened properties are investigated and presented. SCC mix were prepared by varying sand fines content from 6.9% in sand to 0 % and all other mix constituent were kept constant. Fresh properties like slump flow, T<sub>500</sub> time and hardened properties like compressive strength were found.

**Index Terms** – Self compacting concrete, Sand fines, slump flow, compressive strength.

## I. INTRODUCTION

Self compacting concrete is becoming popular day by day due to its ease of use, less voids due to self compactibility and many other advantages. But at the same time SCC is very sensitive to variation of its mix constituent. As shown in figure powder content is more in SCC comparing to conventional concrete. Any particles finer than 125 micron are considered in powder. This fine material affects the flowability, segregation resistance and strength of the concrete. In this paper effect of finer particles present in fine aggregate on properties of SCC is investigated.

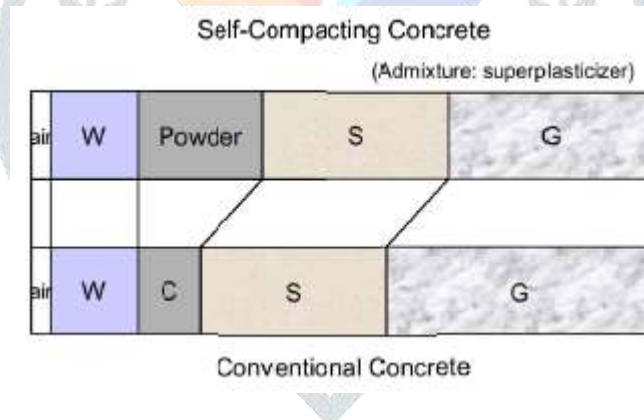


Figure 1. Comparison of material proportion for SCC and Conventional concrete (Okamura<sup>1</sup>)

## II. LITERATURE REVIEW

### 1. EFNARC Guidelines- European Approach

Self compacting concrete consists of almost same constituent materials as conventional concrete, which are cement, aggregates, water and with the addition of super-plasticizer and mineral admixtures (fly ash, Micro silica fume, GGBS, lime stone powder etc.) in different proportions. Usually, the chemical admixtures used are high-range water reducers (HRWR) and viscosity-modifying agents (VMA), which change the properties of concrete.

- ❖ Mineral admixtures are used as an extra fine material, besides cement, and in some cases, they replace cement.
- ❖ High volume of super-plasticizer is used for better workability
- ❖ The high powder content is used as “lubricant” for the coarse aggregates to flow.
- ❖ viscosity-agents to increase the viscosity of the concrete and segregation resistance

Table-1 Requirements of SCC mix in Fresh State as per EFNARC guideline

|                        | Classes | Range (mm)                | Application  | Purpose  | Remarks  |
|------------------------|---------|---------------------------|--|--|--|
| Slump Flow             | SF-1    | 550-650                   | Unreinforced                                       | To check segregation, filling ability, viscosity | If paste extend several millimetres from the C.A. and if C.A. segregated at central area |
|                        | SF-2    | 660-750                   | Normal   |  |  |
|                        | SF-3    | 760-850                   | Very Congested Stru.                               |  |  |
| T 500 time             | VS-1    | $\leq 2$                  | Good Filling, Surface finish                       | Viscosity  | Prone to bleeding and segregation  |
|                        | VS-2    | $> 2$                     | High segregation Resistance                        |  | Lake in Surface finish   |
| V Funnel               | VF-1    | $\leq 8$                  | Same as T500                                       | Viscosity  | Same as T500   |
|                        | VF-2    | 9 to 25                   |  |  |  |
| L-Box                  | PA-1    | $\geq 0.80$ with 2 rebars | Housing/Vertical Structures ( Gap 80 mm to 100 mm) | Passing Ability- Flow without blocking           | No need when gap is more than 100 mm   |
|                        | PA-2    | $\geq 0.80$ with 3 rebars | Civil Engineering Stru. ( Gap 60 mm to 80 mm)      |  |  |
| Sieve Segregation Test | SR-1    | $\leq 20\%$               | For thin slabs & Vertical Application              | Segregation resistance for higher slump flow     | Flow Distance $< 5m$ Confinement Gap $> 80mm$  |
|                        | SR-2    | $\leq 15\%$               | Tall Vertical Application                          |  | Flow Distance $> 5m$ Confinement Gap $< 80mm$  |

SCC requirements are specific to site condition. If reinforcement is little or no, no need of passing ability

#### Salient Point of Mix Design

1. Water/powder ratio by volume of 0.85 to 1.1 by Volume
2. Total powder content - 160 to 240 litres (380-600 kg) per cubic meter.
3. Total Paste Content- 300 to 380 litre
4. Coarse aggregate content normally 27 to 36 per cent by volume of the mix.(750-1000 kg)
5. Water Content 150-210 litre/m<sup>3</sup>
6. The sand content 48-55 % of total aggregate weight.

### III. EXPERIMENTAL INVESTIGATION

#### MATERIAL PROPERTIES

In present study material used are 20 mm coarse aggregate, fine aggregate, PCE based super plasticizer Master Glenium Sky BASF 8549. All imported properties of these materials are found and presented.

Table-2 Specific gravity and Water Absorption of materials used

| Material                 | Specific Gravity | Water Absorption |
|--------------------------|------------------|------------------|
| Cement (PPC)             | 2.9              | -----            |
| Coarse Aggregate (20 mm) | 2.86             | 1.11             |
| Fine Aggregate           | 2.57             | 1.89             |
| Master Glenium Sky 8549  | 1.1              | -----            |

Table-3 Sieve Analysis of Fine Aggregate

| Sr. No. | I.S. Sieve Size | RETAINED |               |                   | % ge Passing | Required % age Passing for Zone-II as per IS-383 | Required % age Passing for Zone-III as per IS-383 |
|---------|-----------------|----------|---------------|-------------------|--------------|--|---|
|         |                 | Weight   | % ge retained | Cum % ge retained |              |  |   |
| 1       | 10              | 0        | 0             | 0.00              | 100          | 100  | 100   |
| 2       | 4.75            | 21       | 2.1           | 2.10              | 97.9         | 90 to 100  | 90 to 100   |
| 3       | 2.36            | 73       | 7.3           | 9.40              | 90.6         | 75 to 100  | 85 to 100   |
| 4       | 1.18            | 168      | 16.8          | 26.20             | 73.80        | 55 to 90   | 55 to 90  |
| 5       | 0.6             | 82.0     | 8.2           | 34.40             | 65.6         | <b>35 to 59</b>                                  | <b>60 to 79</b>                                   |
| 6       | 0.3             | 351      | 35.1          | 69.50             | 30.5         | 8 to 30  | 12 to 40  |
| 7       | 0.15            | 236      | 23.6          | 93.10             | 6.9          | 0 to 10  | 0 to 10   |
| 8       | 0.075           | 69.0     | 6.9           | 100.00            | 0            | 0 to 3   | 0 to 3  |
| 9       | Pan             | 0        | 0             | 100.00            |              |  |   |

Table 4 shows the mix proportion taken in this study. Total 10 mixes are prepared keeping aggregate volume constant to check the effect of aggregate. At first four mixes were prepared by varying sand proportion in total aggregate and after that six mixes were prepared by varying sand fines. Slump flow, T500 time and V funnel test were carried out for fresh concrete and compressive strength test carried out for hardened concrete.

**Table-4 Proportions finalized for SCC mix per cubic meter of concrete:**

|                     | Water | Cement | Admixture-Super Plasticizer (SP) | sand    | 10 mm | 20 mm  | Marble Dust |
|---------------------|-------|--------|----------------------------------|---------|-------|--------|-------------|
| <b>Volume (ltr)</b> | 200   | 137.93 | 2.91                             | 639.16  |       |        |             |
| <b>Kg</b>           | 200   | 400    | 3.2                              | 1107    | 0     | 596.08 | 0           |
|                     | FIX   |        |                                  | VARYING |       |        |             |

**Table 5: Design Mix using Maximum Size of aggregate 20 mm**

| Mix Designation | Combination of Aggregate |       | 20 mm  | 10 mm | Sand    | Marble Dust | Cement | water  | SP    |
|-----------------|--------------------------|-------|--------|-------|---------|-------------|--------|--------|-------|
|                 | FA                       | 20 mm |        |       |         |             |        |        |       |
| Mix 1           | 65:35                    |       | 596.08 | 0.00  | 1107.00 | 0.00        | 400.00 | 200.00 | 3.200 |
| Mix 2           | 60:40                    |       | 684.83 | 0.00  | 1027.25 | 0.00        | 400.00 | 200.00 | 3.200 |
| Mix 3           | 55:45                    |       | 774.53 | 0.00  | 946.65  | 0.00        | 400.00 | 200.00 | 3.200 |
| Mix 4           | 52:48                    |       | 828.81 | 0.00  | 897.87  | 0.00        | 400.00 | 200.00 | 3.200 |

In above mix it is observed that the combination in which sand proportion is more exhibits the lesser strength. After close analysis it was found that fines content in sand used in this mix is about 6.9 %. For combination of 65% sand with 35% of 20 mm aggregate, sand fines is 76.38 kg/m<sup>3</sup>, which comes 4.49% of total aggregate.. Maximum strength in design mix with MSA 20 mm was found for the aggregate combination of 52% sand and 48% coarse aggregate i.e for Mix-4, in which sand fines content was 3.59% of total aggregate i.e. 61.95 kg per cubic meter of concrete. Now to check effect of sand fines, sand fines in aggregate combination of 60%:40 % and 55%:45% (FA to 20 mm CA) reduced from 61.95 kg/m<sup>3</sup> to 0 kg/m<sup>3</sup>. Actually in 60:40 and 55:45 combination sand fines are 70.88 kg/m<sup>3</sup> and 65.32 kg/m<sup>3</sup> respectively.

**Table 6: Design Mix to check Effect of Sand Fines**

| Mix Designation | Combination of Aggregate |       | 20 mm  | 10 mm | Sand (>150 micron) | Sand (<150 micron) | Cement | water  | SP    |
|-----------------|--------------------------|-------|--------|-------|--------------------|--------------------|--------|--------|-------|
|                 | FA                       | 20 mm |        |       |                    |                    |        |        |       |
| Mix 5           | 60:40                    |       | 684.83 | 0.00  | 965.30             | 61.95              | 400.00 | 200.00 | 3.200 |
| Mix 6           | 60:40                    |       | 684.83 | 0.00  | 997.25             | 30                 | 400.00 | 200.00 | 3.200 |
| Mix 7           | 60:40                    |       | 684.83 | 0.00  | 1027.25            | 0                  | 400.00 | 200.00 | 3.200 |
| Mix 8           | 55:45                    |       | 774.53 | 0.00  | 884.70             | 61.95              | 400.00 | 200.00 | 3.200 |
| Mix 9           | 55:45                    |       | 774.53 | 0.00  | 916.65             | 30                 | 400.00 | 200.00 | 3.200 |
| Mix 10          | 55:45                    |       | 774.53 | 0.00  | 946.65             | 0                  | 400.00 | 200.00 | 3.200 |

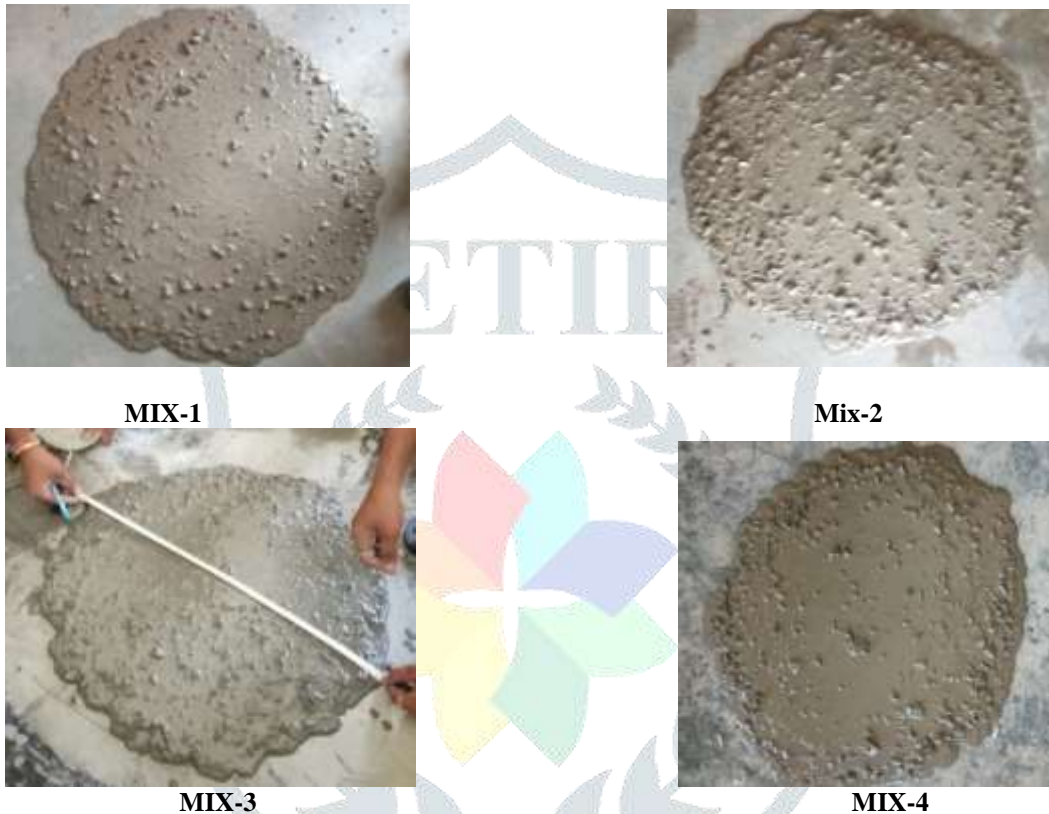


**Figure 2: Fines sieved from sand**

## IV. RESULTS AND DISCUSSION

**Table 7: Compressive strength and fresh properties of design mix with MSA 20 mm**

| Mix Designation | Combination of Aggregate |       | Slump Flow (mm) | T500 (Sec) | V Funnel (Sec) | Compressive Strength |         | Remarks/ Observation |
|-----------------|--------------------------|-------|-----------------|------------|----------------|----------------------|---------|----------------------|
|                 | FA                       | 20 mm |                 |            |                | 7 Days               | 28 Days |                      |
| Mix 1           | 65:35                    |       | 730             | 2.36       | 4.8            | 22.07                | 34.82   |                      |
| Mix 2           | 60:40                    |       | 680             | 2.57       | 4.4            | 22.2                 | 35.52   |                      |
| Mix 3           | 55:45                    |       | 640             | 3.12       | 2.4            | 23.46                | 39.92   |                      |
| Mix 4           | 52:48                    |       | 650             | 3          | 1.66           | 26.9                 | 41.37   | Bleeding             |

**Figure 3: Slump flow for Mix 1, 2, 3 and 4**

It can be seen that, Mix-1, Mix-2 and Mix-3, fine aggregate percentage in total aggregate is 65%, 60% and 55% respectively. Slump flow is decreasing as sand percentage is decreased and strength is increased as sand percentage is decreased even if all other materials are in same amount in the mix.

**Table 8: Compressive strength and fresh properties of design mix varying the sand fines**

| Mix Designation | Combination of Aggregate |       | Slump Flow (mm) | T500 (Sec) | V Funnel (Sec) | Compressive Strength |         | Remarks/ Observation |
|-----------------|--------------------------|-------|-----------------|------------|----------------|----------------------|---------|----------------------|
|                 | FA                       | 20 mm |                 |            |                | 7 Days               | 28 Days |                      |
| Mix 15          | 60:40                    |       | 680             | 1.59       | 7.77           | 21.3                 | 39.36   |                      |
| Mix 16          | 60:40                    |       | 700             | 1.72       | 6.5            | 23.29                | 39.19   |                      |
| Mix 17          | 60:40                    |       | 675             | 1.36       | 4.8            | 23.99                | 41.49   |                      |
| Mix 18          | 55:45                    |       | 675             | 1.57       | 8.4            | 17.6                 | 39.84   | Bleeding             |
| Mix 19          | 55:45                    |       | 700             | 1.51       | 7.5            | 19.14                | 40.02   | Bleeding             |
| Mix 20          | 55:45                    |       | 700             | 1.34       | 6.99           | 23.7                 | 41.81   | Bleeding             |



Figure 3: Slump flow for Mix 4, 5 and 6



Figure 4: Slump flow for Mix 7, 8 and 9

**Discussion of Result:** It was found that if sand fines are decreased the strength of mix is increased but chance of bleeding increases. In above mix strength was almost becomes equal to that with the maximum strength achieved in Mix 3, without much compromising to slump flow.

#### IV. CONCLUSION

1. Strength of SCC mix is affected by the amount of sand fines present in mix. Strength is increased as sand fine is decreased.
2. Sand used in present study has around 6.9 % particles finer than the 150 micron sieve. Sand fines shall be under 4% for SCC mix with 20 MSA.
3. It can be concluded that for lower grade of concrete in which Cementitious material is less effect of sand fines is considerable.
4. Fines are necessary for viscosity improvement and to stop bleeding, but sand fines present in mix decreases the strength.

#### REFERENCES

- [1] Okamura H, Ouchi M. Self-compacting concrete. *Journal of advanced concrete technology*. 2003;1(1):5-15.
- [2] BIBM C, EFCA E. ERMCO. *The European guidelines for self-compacting concrete: specification, production and use*. 2005 May.
- [3] Brouwers HJ, Radix HJ. Self-compacting concrete: the role of the particle size distribution. In *First International Symposium on Design, Performance and Use of SCC, Hunan, China 2005* (pp. 109-118).
- [4] Rodriguez de Sensale G, Rodriguez Viacava I, Aguado A. "Simple and rational methodology for the formulation of self-compacting concrete mixes." *Journal of Materials in Civil Engineering*. 2015 Jul 31;28(2):pp.04015116-1-10
- [5] Shi C, Wu Z, Lv K, Wu L. "A review on mixture design methods for self-compacting concrete. *Construction and Building Materials*." 2015 Jun 1;84:387-98.
- [6] Wang X, Wang K, Taylor P, Morcoux G. "Assessing particle packing based self-consolidating concrete mix design method." *Construction and Building Materials*. 2014 Nov 15;70:439-52.

- [7] D'Souza B, Yamamiya H. "Applications of smart dynamic concrete. InThird International Conference on Sustainable" Construction Materials and Technologies 2013 Aug 18 (pp. 18-21).
- [8] Su N, Hsu KC, Chai HW. A simple mix design method for self-compacting concrete. Cement and concrete research. 2001 Dec 1;31(12):1799-807.
- [9] Radhika KL, Kumar PR, Chand MS, Prasad B. Optimization of Mix Proportioning for Self Compacting Concrete using Particle Packing Theories. Washington DC, USA 15-18 May 2016 Edited by Kamal H. Khayat. 2016 May 15:53.
- [10]Hwang CL, Hung MF. Durability design and performance of self-consolidating lightweight concrete. Construction and building materials. 2005 Oct 1;19(8):619-26.
- [11]Bhattacharya A, Ray I, Davalos J. Effects of aggregate grading and admixture/filler on self-consolidating concrete. Open Construction and Building Technology Journal. 2008;2:89-95.
- [12]Shrivastava AK, Kumar M. Compatibility issues of cement with water reducing admixture in concrete. Perspectives in Science. 2016 Sep 1;8:290-2.
- [13]Roussel N, Le Roy R. The Marsh cone: a test or a rheological apparatus?. Cement and Concrete Research. 2005 May 1;35(5):823-30.
- [14]De Larrard F. Concrete mixture proportioning: a scientific approach. CRC Press; 2014 Apr 21.
- [15]Neville AM. Properties of concrete. London: Longman; 1995 Jan.
- [16]Shetty MS. Concrete technology. S. chand & company LTD. 2005 May:420-53.

