# Analysis And Design Of Cable Suspended Bridge Along With Identify Behaviour Of Cables During Moving Loads

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**Abstract:** Cable-stayed bridges have emerged as the dominant structural system for long span bridge crossings during the past thirty years. That success is due to a combination of technical advancements and pleasing aesthetics attributes. The interaction of the various structural components results in an efficient structure which is continuously evolving and providing new methods to increase span lengths. The objective of this thesis is to describe in detail the basic structural behaviour of cables under moving loads cable-stayed bridges, and to present the lack of fit forces of cables during construction stage and under moving load situation

Keywords: cable-stayed bridge, truss model, incremental loading, sag effect, Ernst equivalent elasticity modulus, pylon, deck, seismic inertia loads, geometric stiffness, slackening.

### I. Introduction

Cable stayed bridges are more popular because of there feasibility and aesthetic appearances at various locations and as of they are suitable for greater lengths and also they are more reliable structural system easy erection. In the preliminary design of a cable-stayed bridge, the required pretensions and sections of the cables can be pre-estimated by hand, by considering the steps of erection of the bridge, that is the suspension of successive parts of the deck from the inclined cables.

#### **CABLES**

The four types of strand configuration are

Sr. No.		Ultimate Tensile Stress	Young Modulus(σ)		
		(E) N/mm2	N/mm2		
A.	Helically-wound galvanized strands.	670	165 000		
B.	Parallel wire strands	1860	190 000		
C.	Strands of parallel wire cables.	1600	200 000		
D.	Locked coil strands.	1500	170 000		
	Ultimate Tensile Stress.				

## **METHODOLOGY**

initial cable pretension analysis starts	77							
		Constru	Construction stage analysis					
Final stage analysis		con	nsidering ca	mber		Adjust cable pretension		
Assign constraints		Compa	ro final cab	lo toncion		- 8		
9		Compare final cable tension and design cable tension		$\vdash$	NG	-		
satisfying initial	100	and de				ING		
equilibrium state		PERAL PROPERTY.		ok	-		-	
		Verify cable tension		_				
Construction		-	at each sta	ge		NG		
stage analysis	Adjust cable							
	pretension			ok				
Verify member		Verify member forces at each stage						
forces	NG			NG				
ok				ok				
Specify design cable	2.0	Compare final displacement and camber		NG Adjust camber for tower and PC girder		er for		
tension						girder		
	- 37		1		- 3			
Generate camber for				ok				
tower and PC girder	- O							
tonici ana rognaci	Adjust	NG	Verify n	nember	ok	Initial	cable pr	etensio
	design cable	22		- N	1	nalysis ends		

GENERAL DATA OF CABLE	STAYED BRIDG	E:				
	END	MID	END	TOTAL		
SPAN LENGTH (m)	40	125	40	205		
		TOP	BOTTOM	TOTAL		
TOWER HEIGHT (m)		40	20	60		
ELEMENT TYPES	DECK & T	DECK & TOWER		BEAM ELEMENT		
	CABI	CABLE		TRUSS ELEMENT		

**CABLE ARRANGEMENT:** Symmetrically arranged with respect to the center of the deck portion **Boundary condition:** Both End Fixed for the pylon

**Unknown Load factors:** from applied load check Influence Matrix **Cable Force Tuning** for initial pretension in cables

## On Site Construction installation stages

- a) Girder Installation
- b) Cable Installation with First Tensioning
- c) Slab casting
- d) Slab hardening and Secondtensioning

## THE ANALYSIS

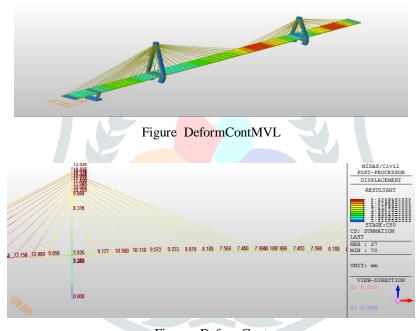


Figure : DeformCont

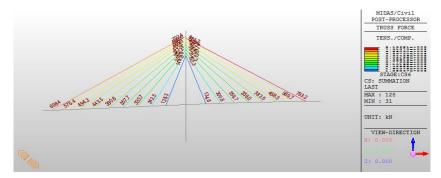
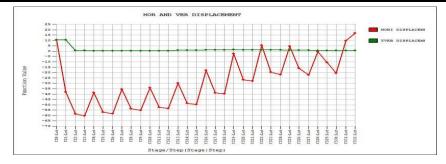


Figure Truss Force.



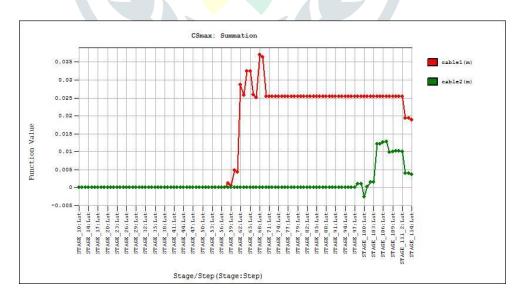
Horizontal and Vertical displacement



Variation Of Stress During Construction Stage



Bending Moment Tower And Girder



Cable Pressure

### II. RESULTS

In cable stayed bridges as a unit pretension load applied in cables we get unknown load factors through influence matrix and then apply unknown load factors to find cable tuning forces. Also construction stage analysis as construction stage move on we consider time duration and load effects. As in construction forward and backward stage analysis cable forces are initially controlled. cable behavior are reliable and safe during moving load as we analysed previous calculations.

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